Stream Crossing Alternatives

July 2, 2012

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California Department of Forestry

and Fire Protection



Outline

- Introduction
- II. Hydrology Basics Related to Crossing Design
- III. Types of Crossings Available
- IV. What Can Go Wrong at Crossings?
- V. Proper Design and Construction for New or Replacement Crossings
- VI. Summary—Take Home Messages
- VII. References for Stream Crossings

I. Introduction

- Why are stream crossings a BIG deal?
- Monitoring results tell us that crossings often have problems (~20%) and that a high percentage of sediment delivery to streams occurs at or near crossings.
- Crossings are built in risky locations subject to large environmental stressors.
- Crossings are built with planned failure in mind.
- Crossing structures have an expected life.
- Crossing design, installation, or maintenance is often inadequate.

No such thing as a "permanent" culvert crossing





II. Hydrology Basics Related to Crossing Design

Background Information on Peak Flows (or Flood Flows)



Three Main Climatic Conditions that Produce Peak Flows

- Spring snowmelt season
- High intensity/long duration rain storm
- Rain-on-snow event



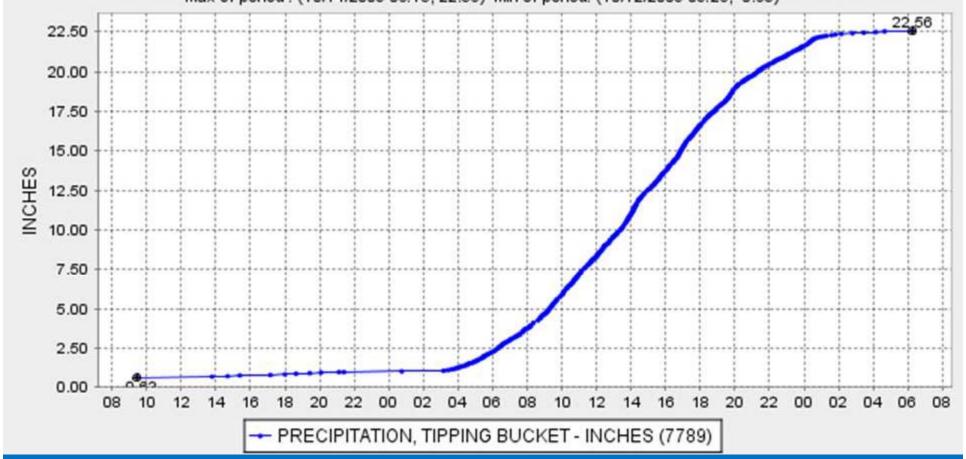
Central Sierra Nevada in the Rainon-Snow Zone

Example of a High Intensity/Long Duration Frontal Rain Storm: October 13, 2009 -- Mining Ridge, Monterey County 21.2 inches in 24 hours. 1000 yr RI rainfall event

MINING RIDGE (MNG)

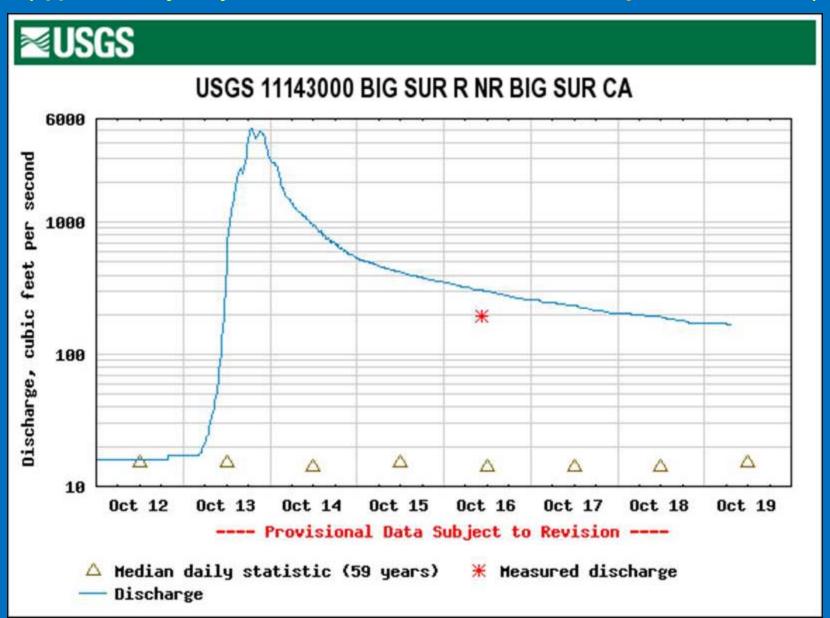
Date from 10/12/2009 08:29 through 10/14/2009 08:29 Duration: 2 days

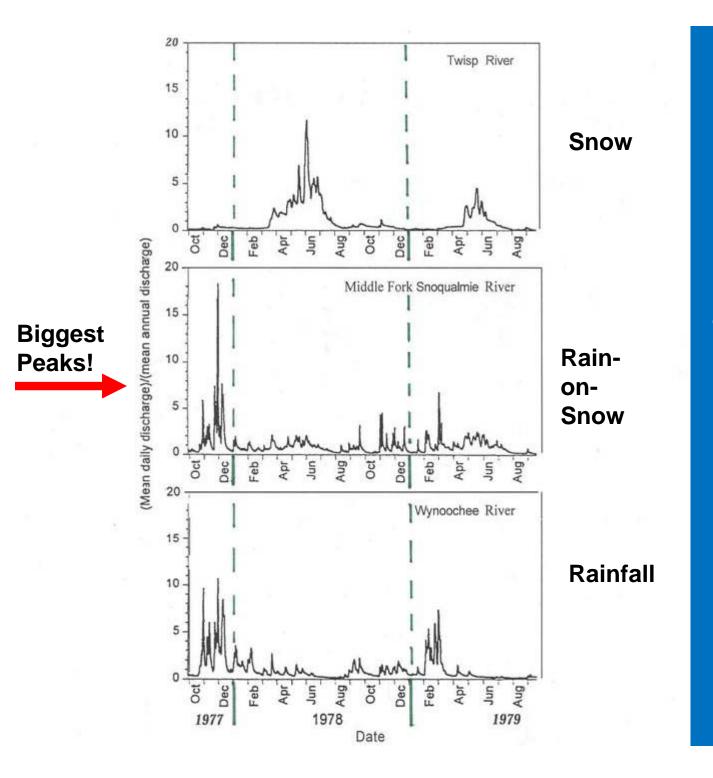
Max of period: (10/14/2009 06:15, 22.56) Min of period: (10/12/2009 09:26, 0.63)



Big Sur River: 17 cfs to 4,830 cfs in 17 hrs!

(approximately 7.5 yr Return Interval stream flow event—dry soils in October!)



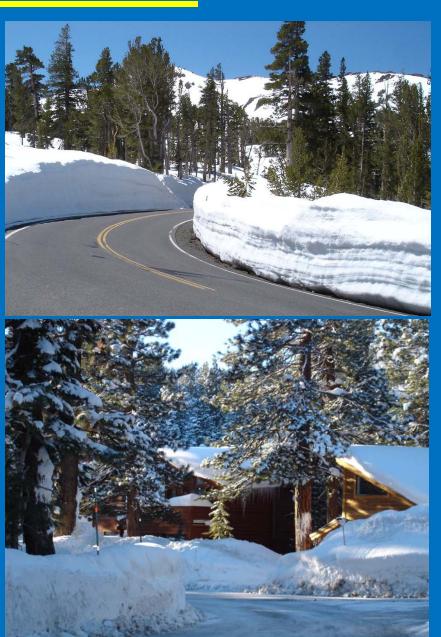


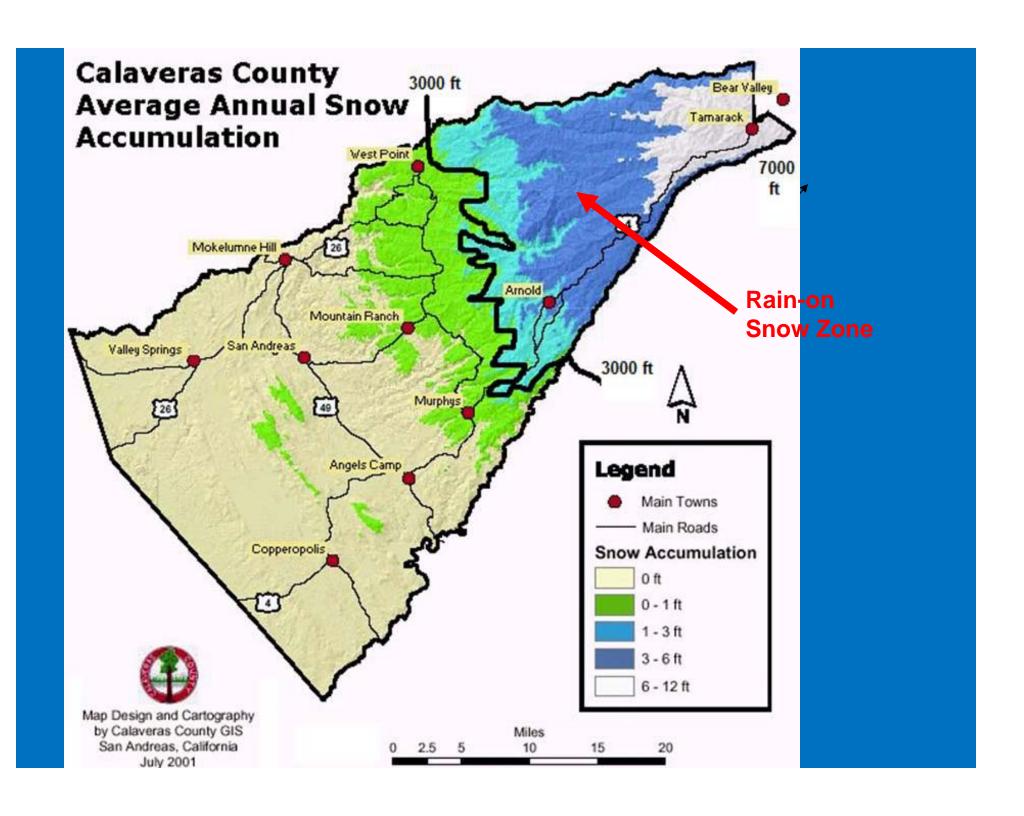
Hydrology Dominated by 3 Different Precipitation Types

Image: Ziemer and Lisle (1998)

Hydrology Basics

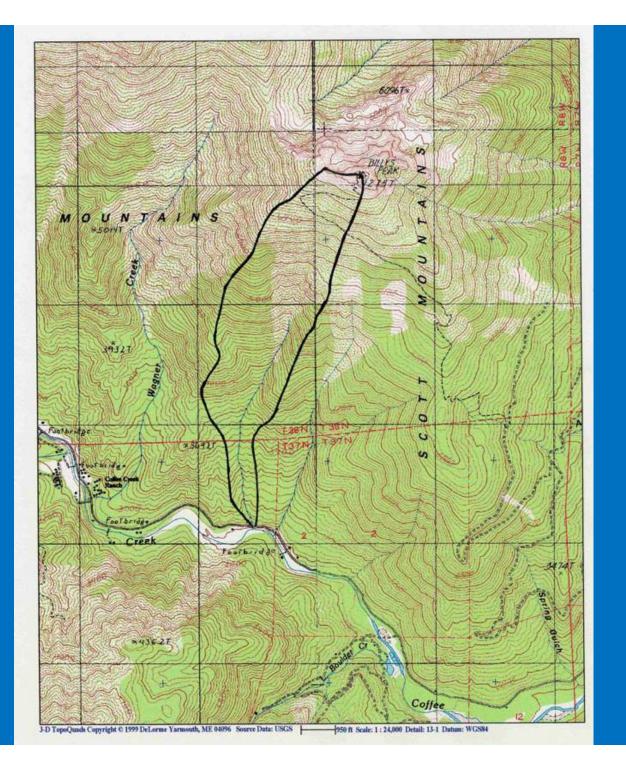
- Rain-on-Snow Zone Area on a mountain range where a snow pack forms that may or may not last through the winter. Rain occurs in this zone several times a year and may melt all the snow, depending on total snowpack depth (McGurk and Cafferata 1991).
- 95% of the major floods in California have involved rain and melting snow!
- Rain-on-Snow Zone for Central Sierra Nevada
 - 3000 to 7200 feet





Key Factors for Predicting Peak Flows for Stream Crossings

- Drainage area above the stream crossing site (acres or mi²).
- Watershed elevation data (ft).
- Precipitation Data.
 - Average annual precipitation (inches)
 - Rainfall intensity in inches/hour for the 100-yr storm event



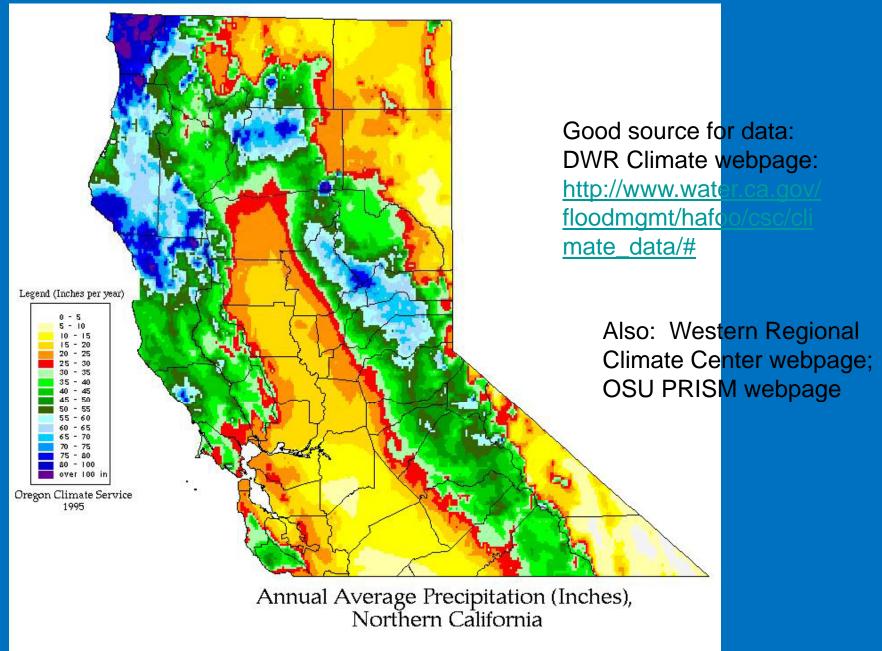
Example for Computing Watershed Area:

Coffee Creek Tributary

Trinity River Watershed

Basin area = 412.5 acres or 0.645 mi²

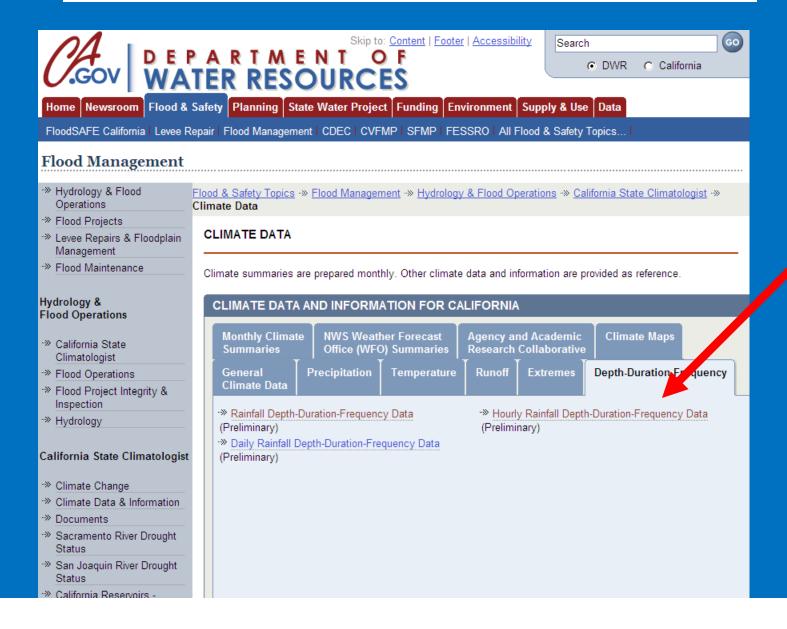
Image: Wopat, CGS



Period: 1961-1990

Rainfall Intensity Data Source

http://www.water.ca.gov/floodmgmt/hafoo/csc/climate_data/#



100 Year RI Rainfall Intensity Data Example: Georgetown

	15 Minute	30 Minute	1 Hour
Duration			
Return Period: 100 Year	0.68 inches	0.91 inches	1.29 inches
Intensity in Inches per Hour	2.72	1.82	1.29

Shorter Duration = Higher Intensity

Source: DWR Climate webpage

California Forest Practice Rule Requirements for Stream Crossings

[Non-Federal Timberlands with Commercial Timber Harvesting]

All permanent watercourse crossings that are constructed or reconstructed shall accommodate the estimated 100-year flood flow, including sediment and debris.

All permanent crossings must allow for upstream and downstream fish passage.

Forest Practice Rule Requirements for Stream Crossings

100-Year Flood Flow

A flood flow that has a 1% annual chance of occurrence in any given year.

Yosemite
- Merced
River

January 1997



Eel River December 1964

Photo: PALCO



Forest Practice Rule Requirements for Stream Crossings

Simple methods to determine 100-yr flood flows based on rainfall and drainage area above crossing site:

- 1. Rational Method
- 2. Regional Regression Equations
- 3. Flow Adjustment by Drainage Area

Complicated Methods: NRCS TR-55, USACE HEC-HMS

1. Rational Method

 $Q_{100} = CIA$ Where:

Q = Discharge in <u>cfs</u> (cubic feet per second)

C = Runoff coefficient (proportion of rainfall that runs off)

I = Rainfall intensity in <u>inches/hour for</u> 100 yr <u>storm event</u>

A = Basin drainage area in <u>acres</u>

Best used for basins ≤ 200 acres

Rational Method

Advantages

- Easy to use
- Widely used and understood
- Accounts for local conditions

Disadvantages

- Hourly rainfall data may be difficult to obtain
- Many assumptions not met
- Uses only rainfall; does not address rain-on-snow events
- For small watersheds only, better for urban areas

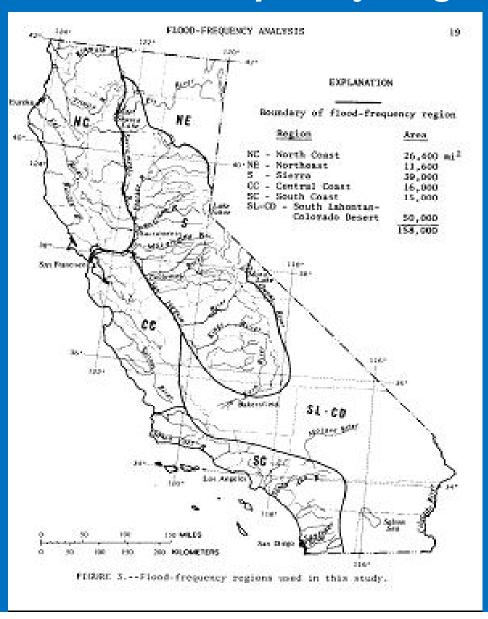
2. Regional Regression Equations

Sierra Region Example: $Q_{100} = 15.7 (A)^{0.77} (P)^{1.02} (H)^{-0.43}$

- <u>Developed by USGS</u> (Waananen and Crippen 1977. USGS WRI 77-21)
- Empirical based on regression of gaged stream discharge versus (>700 stations):
 - basin area (A)
 - average annual rainfall (P)
 - average basin elevation (H)
- State subdivided into 6 areas (flood frequency regions)

Regional Regression Equations

Map of Flood Frequency Regions



Regional Regression Equation Method

Advantages

- Easy to use
- Uses average annual rainfall data
- Based on <u>measured flood flows</u> from widely distributed, numerous locations
- Includes rain-on-snow flood data

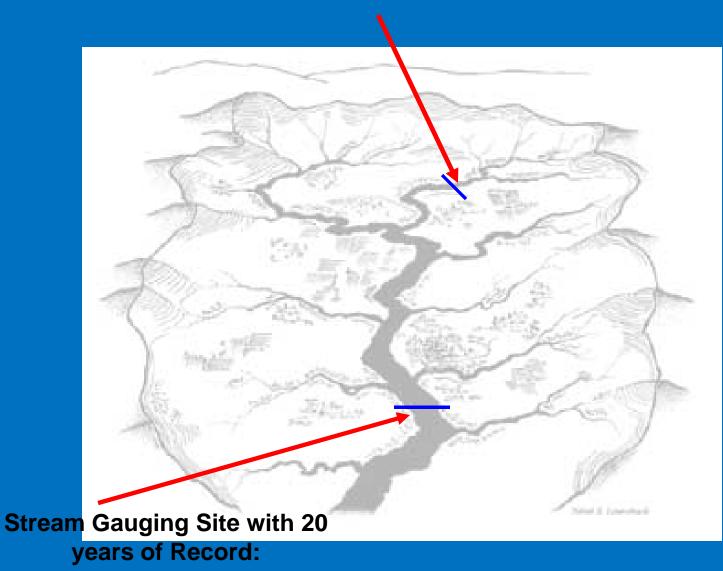
Disadvantages

- Broad-brush method (entire state covered by only 6 regions)
- Tends to overestimate in some areas, underestimate in others
- Based on old (1977) data

3. Flow Adjustment by Drainage Area Method

- Used for an <u>ungaged</u> basin when discharge data from a downstream station or nearby gaged (hydrologically similar) site is available.
- Adjusts 100-year discharge from the gaged basin for the difference in drainage area between the ungauged basin and the gauged basin by using a simple equation.

Crossing Site: Drainage Area = 10 sq miles



years of Record: **Drainage Area = 100 sq miles**

Flow Adjustment by Drainage Area Method

$$Q_{100u} = Q_{100g} (A_u/A_g)^b$$

where:

- Q_{100u} = 100-year discharge at ungaged site
- Q_{100q} = 100-year discharge at gaged site
- A_u = drainage area of ungaged site
- A_q = drainage area of gaged site
- b = exponent for drainage area from the appropriate USGS Regional Regression Equation

(= 0.77 for 100-yr equation for Sierra Region)

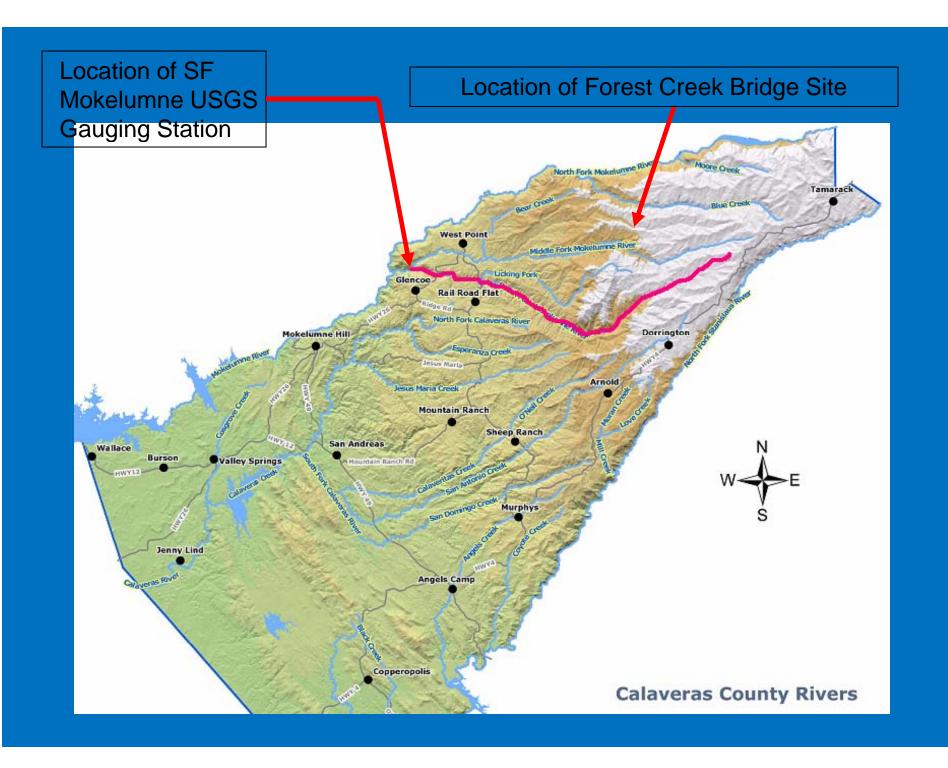
Flow Adjustment by Drainage Area Method

Advantages

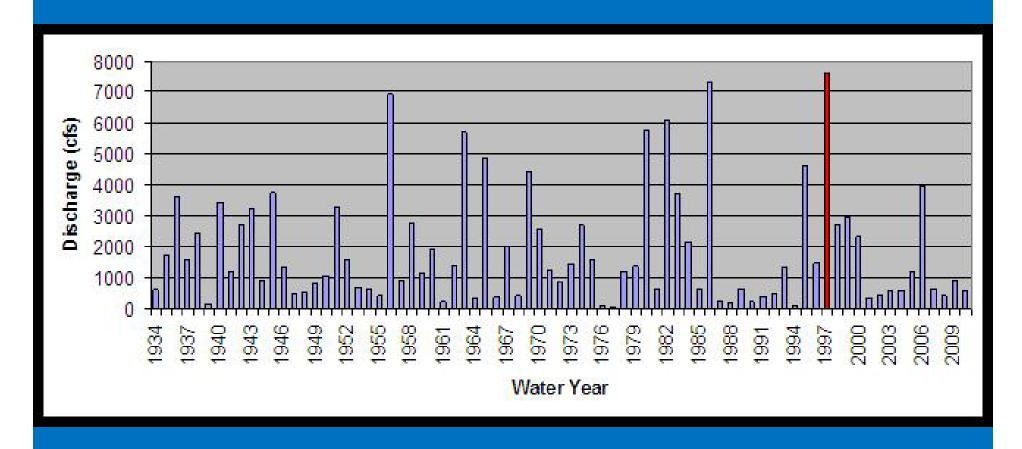
- Easy to use
- Highest confidence, since local data is used

Disadvantages

- Nearby unregulated gauging station data
 (≥ 20 yrs) required.
- Must determine 100-yr flood flow for gauged station.
- Difference in drainage areas should not exceed one order of magnitude.



South Fork Mokelumne River: Peak Discharges: 1934 to 2010



Flood of Record: January 1997–7,610 cfs

South Fork Mokelumne River: Estimated Peak Discharges (1934-2010)

RETURN PERIOD (yr)	STREAMFLOW (cfs)	
2	1,427	
5	2,851	
10	4,428	
25	6,966	
50	9,251	
100	11,870	
200	14,840	
500	19,330	

Use USGS PEAKFQ computer program

Example in the South Fork Mokelumne River Watershed

For a new bridge site on Forest Creek, the 100-year discharge can be estimated as:

 $Q_{100u} = Q_{100g} (A_u/A_g)^b$

= 11,870 cfs $(8.0 \text{ mi}^2/75.1 \text{ mi}^2)^{0.77}$

= 2,116 cfs



Estimating 100-Yr Flood Flow and Sizing Pipes

- 100-yr flood flow is estimated by relationships between precipitation and watershed characteristics and runoff (use multiple predictive, office methods).
- Use a simple nomograph or chart to convert 100-yr discharge to pipe size.
- Check result by making a direct stream channel cross-section field measurement [area of pipe should be at least 3x the bankfull cross sectional area].

Recommend: Minimum of 24 inch pipe for a stream crossing

State of California
The Resources Agency
Department of Forestry & Fire Protection



Designing Watercourse Crossings for Passage of 100-year Flood Flows, Wood, and Sediment

California Forestry Report No. 1 Peter Cafferata, Thomas Spittler, Michael Wopat, Greg Bundros, and Sam Flanagan

February 2004



Guidebook produced in 2004 to assist foresters in designing watercourse crossings and predicting flood flows

Available online at:

http://www.fire.ca.gov/resource_mgt/downloads/100yr32links.pdf

III. Types of Crossings Available

Types of Crossings

- A. Culverts
- **B.** Open Bottom Arches
- C. Bridges
- D. Fords
- E. Temporary Crossings
 - Temporary Fords [e.g., Spittler log fill, rock fill, etc.]
 - Temporary Culverts
 - Temporary Bridges

No Permanent Humboldt Log Crossings

Which One Should I Use? Depends on...

- Watercourse class (e.g., fish present?).
- Watershed drainage area and expected size of 100 yr flood flow.
- Channel slope; landslide susceptibility.
- Maintenance expected.
- Amount and type of traffic expected.
- Road type (permanent, seasonal, temporary).
- Amount of wood and sediment expected to reach the crossing location.
- Topography at the crossing site (incised or flat?).

A. Culverts

- Culverts in forest settings very common (~70% of crossings)--mainly steel or plastic.
- Aluminum <u>not</u> used as much since the mid-1980's—too expensive. At that time, plastic pipes became available.
- Plastic now used heavily (particularly up to 36 inches—especially in the Coastal Mountains).

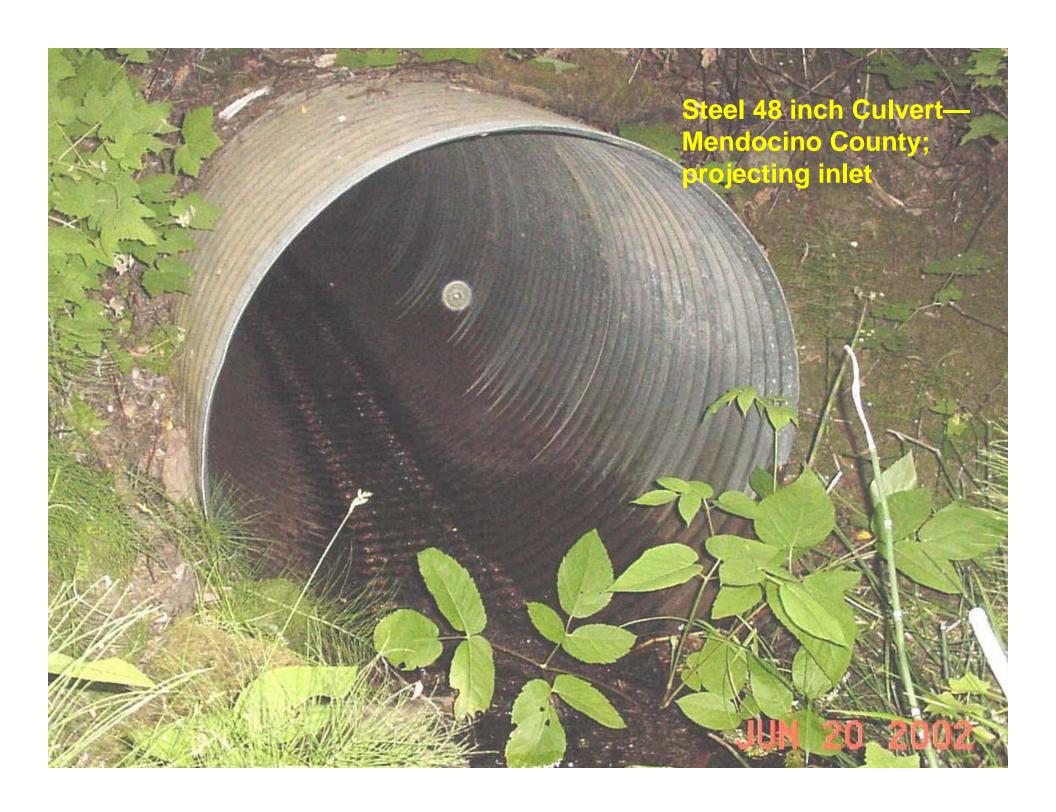
A. Culverts

Pro's

 Good for small, nonfish headwater streams where winter maintenance is possible and will occur.

Con's

- Require lots of maintenance!
- Steel--expected life often only ~25 yrs (range 10-100 yrs).
- Relatively high probability of failure, especially from sediment and woody debris.
- Bad for fish passage.







Plastic Pipe Pro's and Con's

- Caltrans expects a minimum life of 50 years for HDPE pipes exposed to sunlight. UV damage is generally not a concern.
- Benefits that HDPE pipe have over CMP include: (1) light weight, (2)
 ability to be cut with hand tools (saws, chainsaws, etc.) that don't pose
 a large fire hazard, (3) abrasion resistant, and (4) resistant to corrosion
 due to low pH soils.
- <u>Double-walled</u> pipes are very common now and provide:
 - more rigidity to accommodate higher static (overburden) and dynamic traffic loads.
 - a lower roughness (n) value to increase the conveyance capacity of the pipe (about the same as a concrete pipe).
- Problems: (1) increased flow velocities on inclined culverts; and (2) significant energy dissipation structures are often needed below HDPE pipes.
- Biggest problem: <u>Fire Damage</u>.



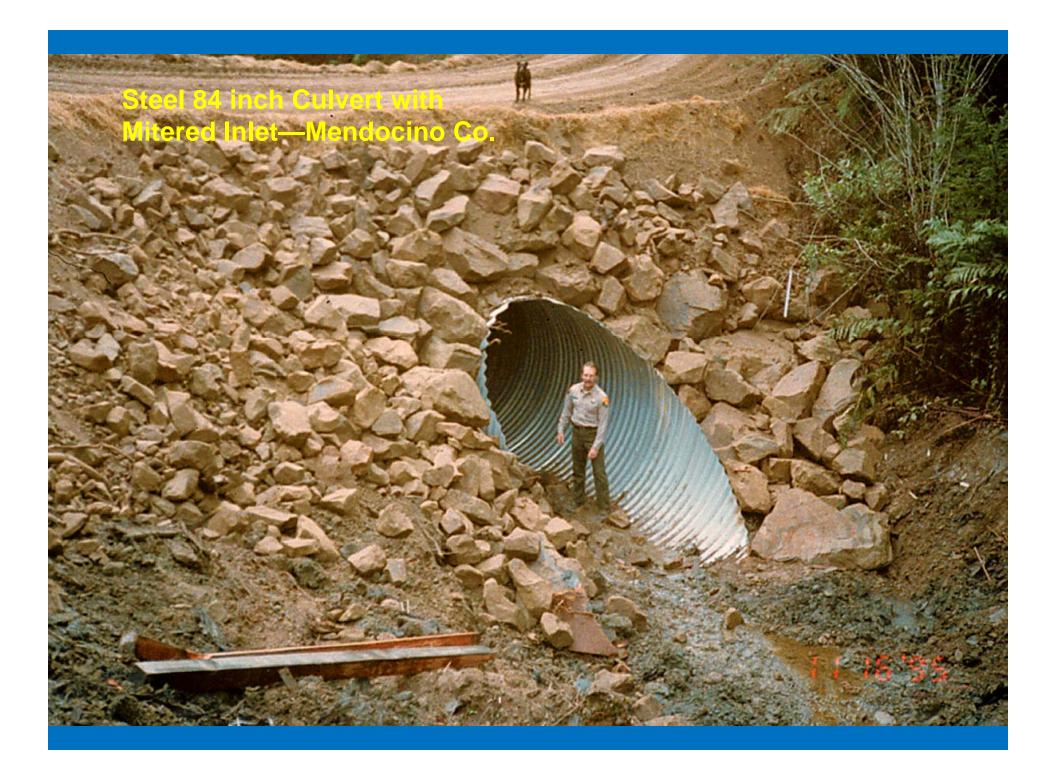
Holes left where plastic culvert burned in small drainages

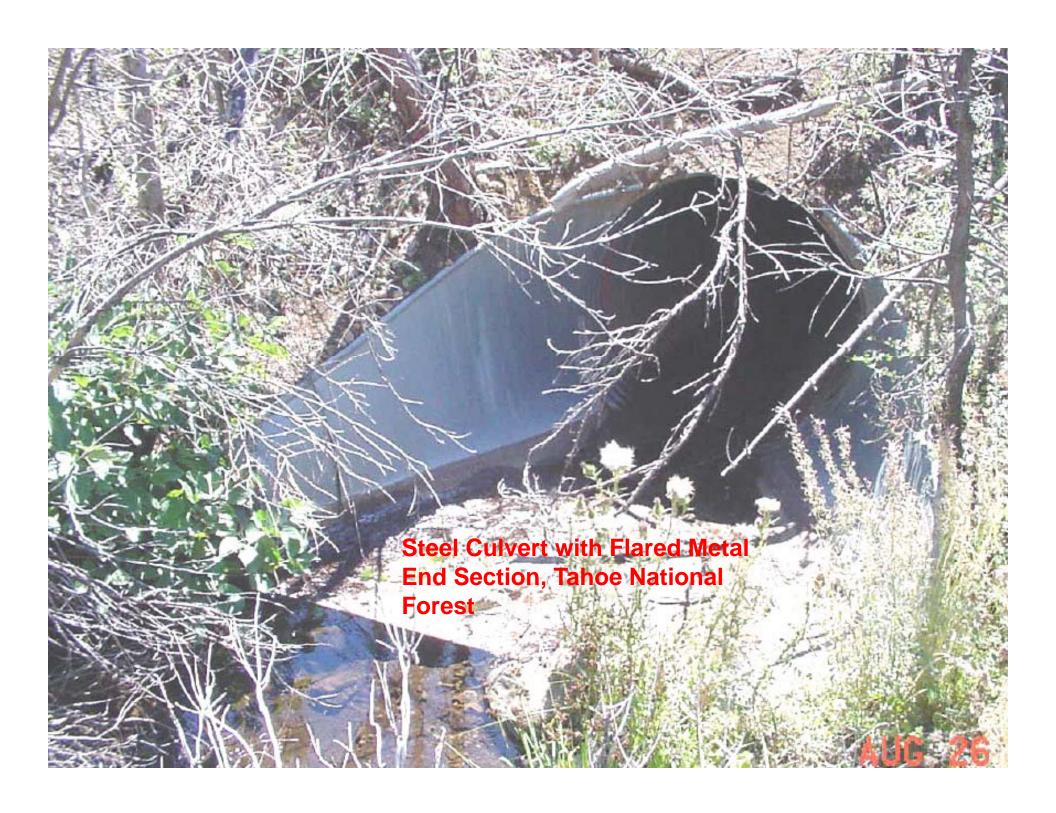
Roca/Rosa 2007 Fires

San Diego County

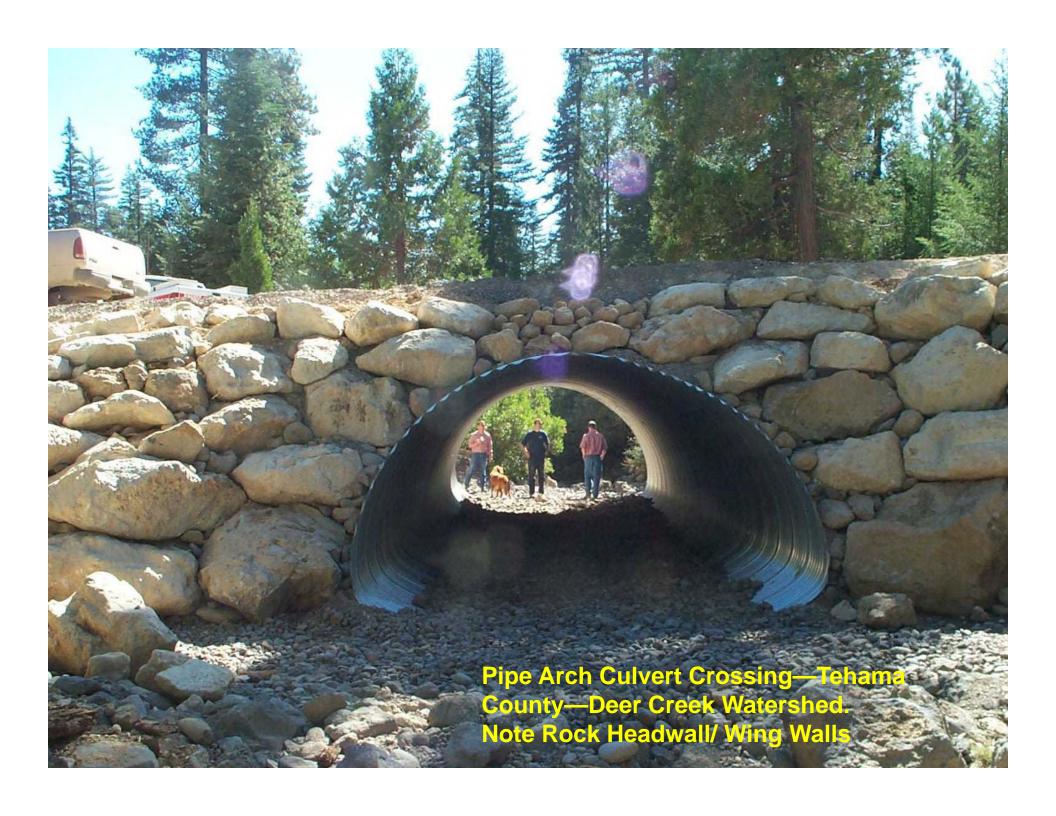
Photo: R. Eliot, CAL FIRE (retired)







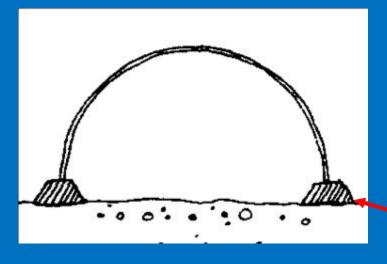




B. Open Bottom Arch

Pro's

- Excellent for fish passage.
- May be cheaper than a bridge.



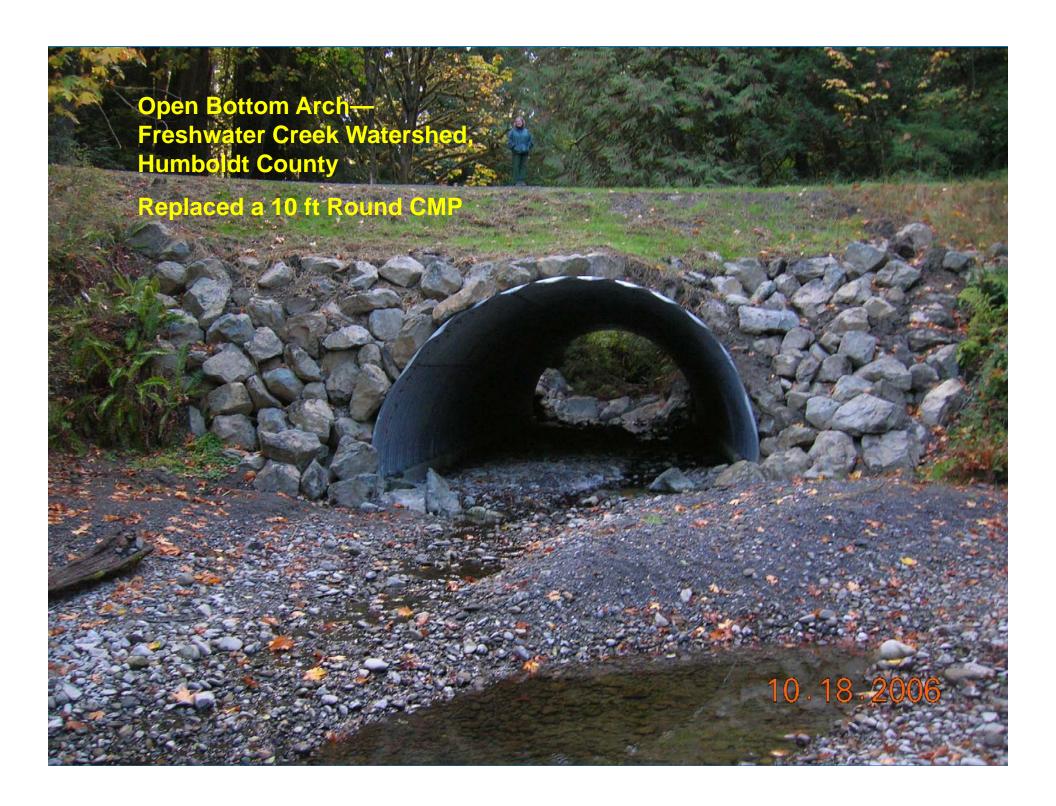
• Con's

- Expensive.
- Will require a professional engineer design.
- Can fail by undermining if concrete footings not on solid rock base.

Concrete footing

Image: Weaver and Hagans 1994





C. Bridges

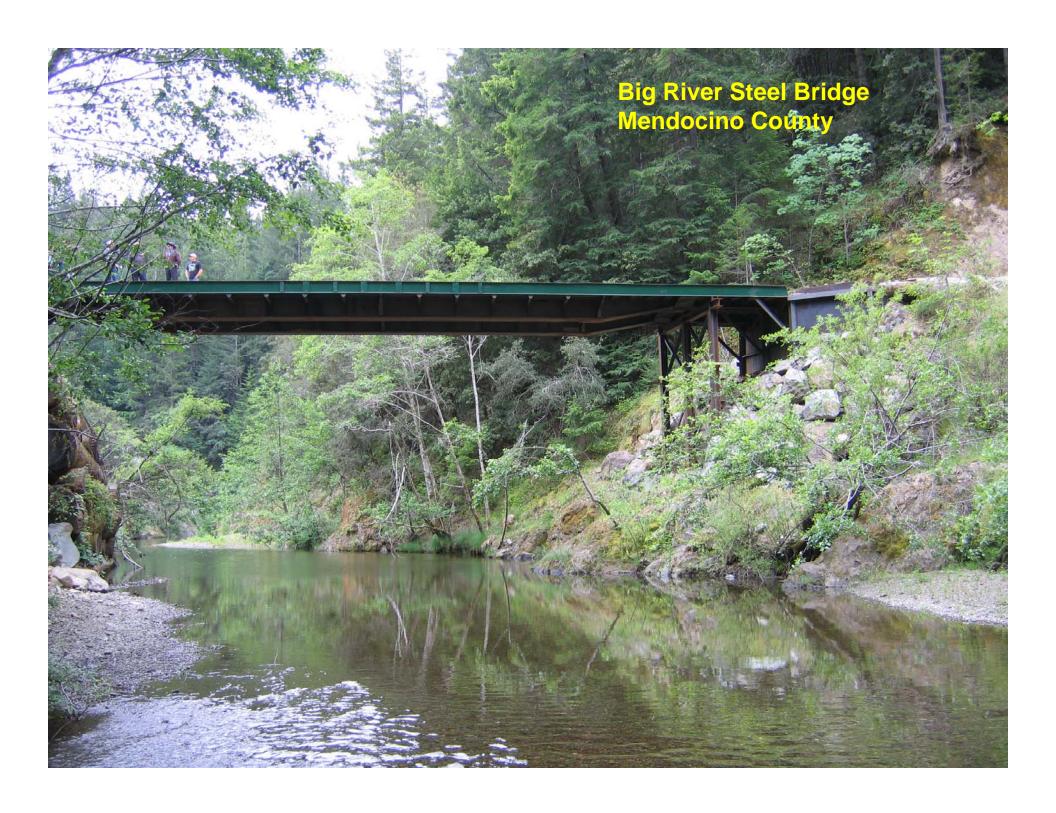
Pro's

- Excellent for fish passage.
- If built correctly, long expected life (low chance of failure).
- Little impact to the stream channel.
- Little sediment entry.
- Low overall environmental impact.
- Good for incised stream channels/larger watercourses.

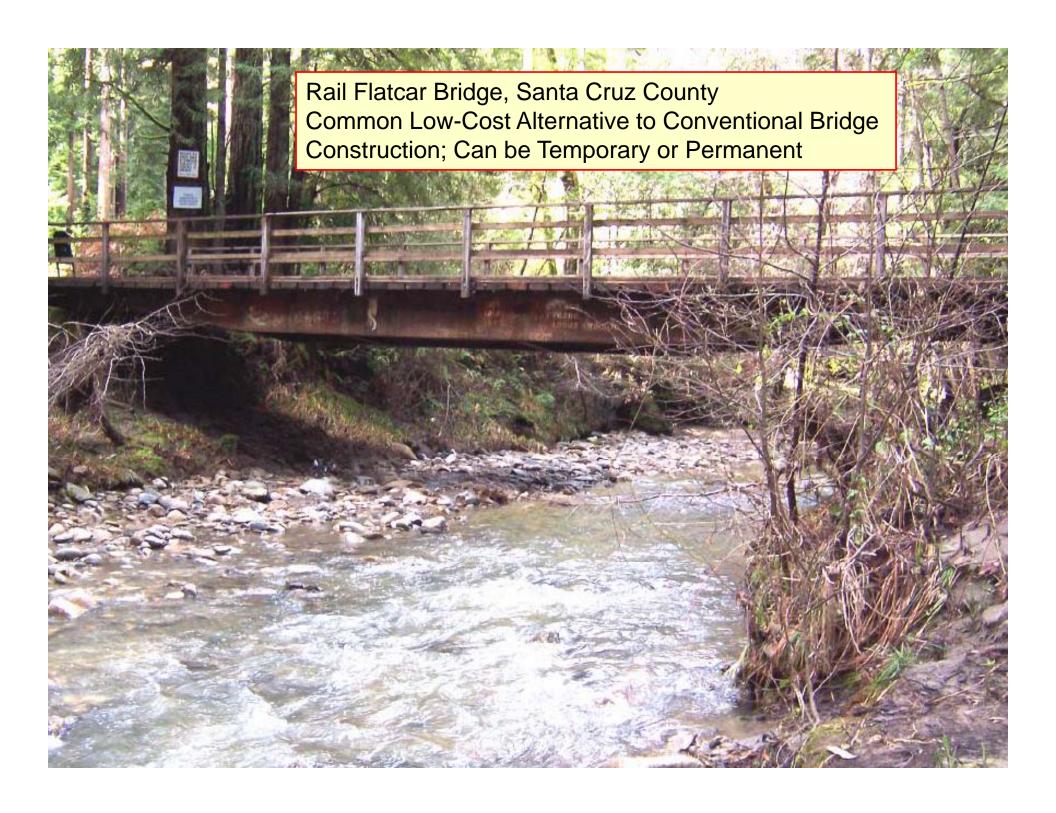
• Con's

- Expensive.
- Railroad flatcar bridges are \$20,000 to \$50,000, depending on length (55 ft or 90 ft) + \$10,000 or more to install).
- May require Professional Engineer design.









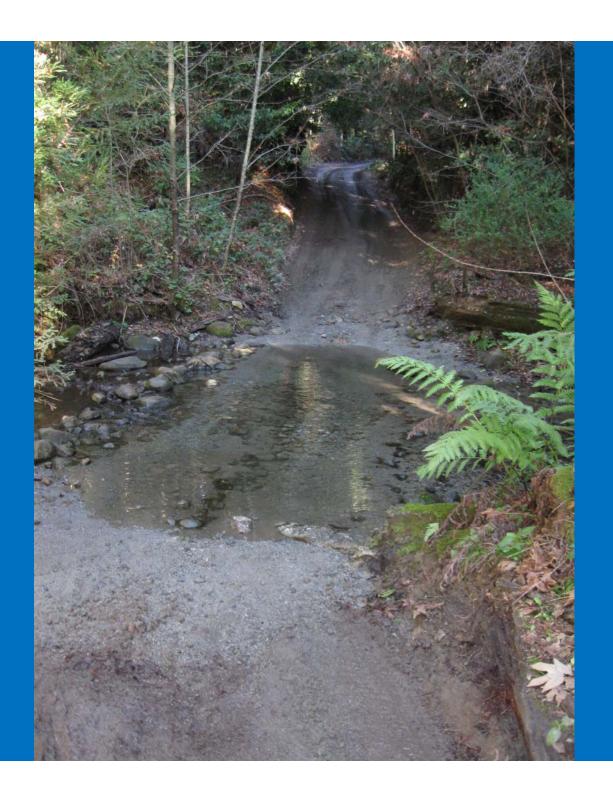
D. Ford Crossings

Pro's

- Often relatively inexpensive alternative for small to medium sized streams w/stable bottoms.
- Low maintenance.
- Low chance of failure if designed correctly.
- Better than a pipe where winter maintenance will not occur (no plugging).
- Not very sensitive to specific flow volumes ("forgiving").
- Good for channels susceptible to landslides/debris flows.

Con's

- Can have high sediment entry, high impact to stream channel, especially with lots of traffic.
- Rock ford crossings can fail easily if not designed correctly.
- Not passable during flood flows!
- Improved ford crossings (concrete slabs) bad for fish passage—prone to scour around edges.



Unimproved Wet Ford Crossing with Chronic Sediment Entry into Hinckley Creek

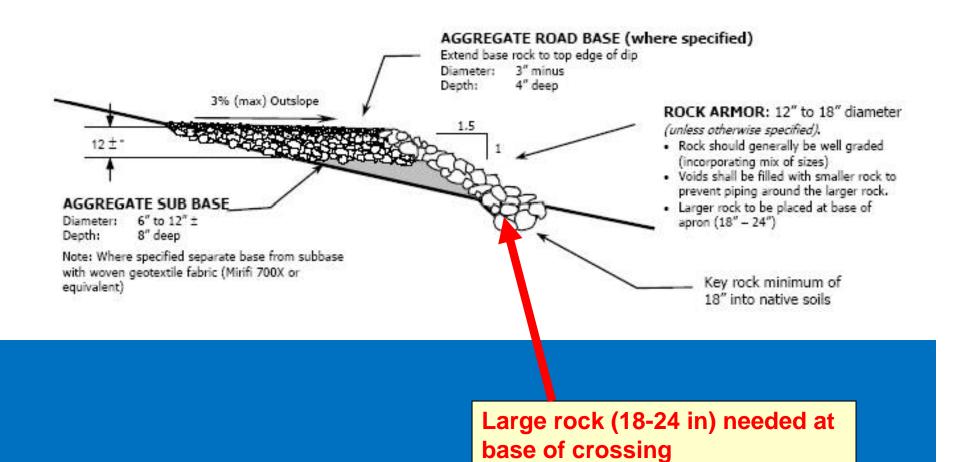
Santa Cruz County

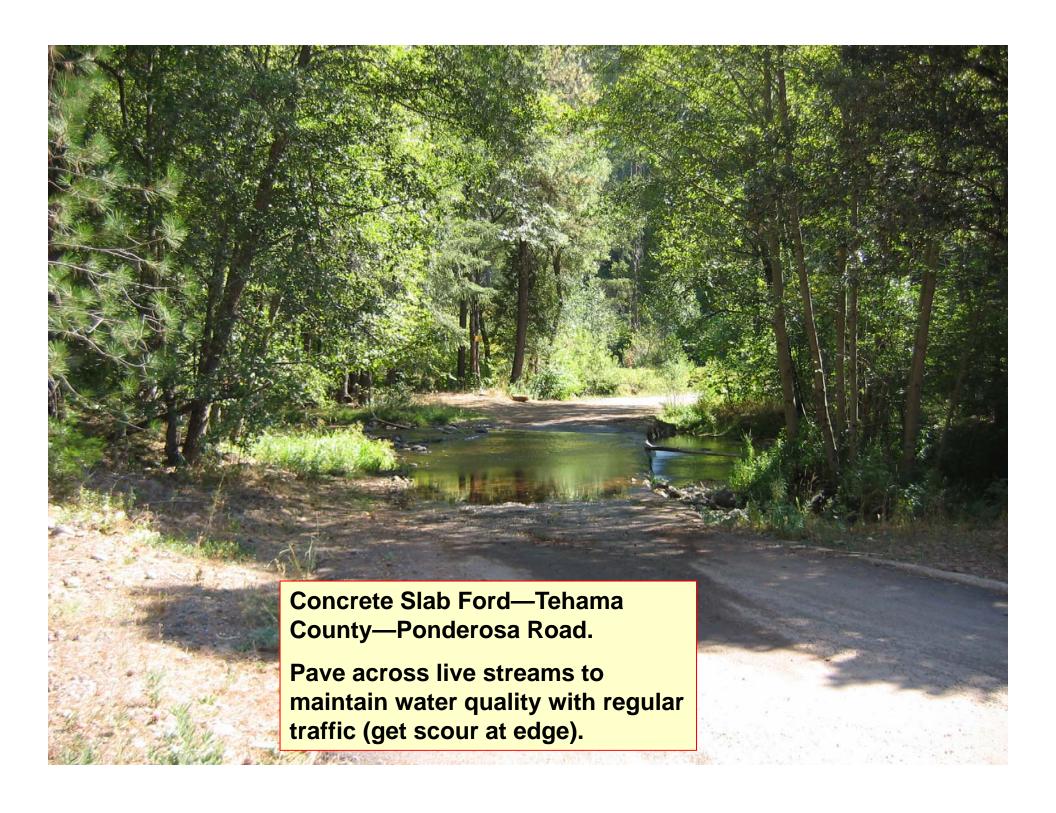
NOT DESIRABLE

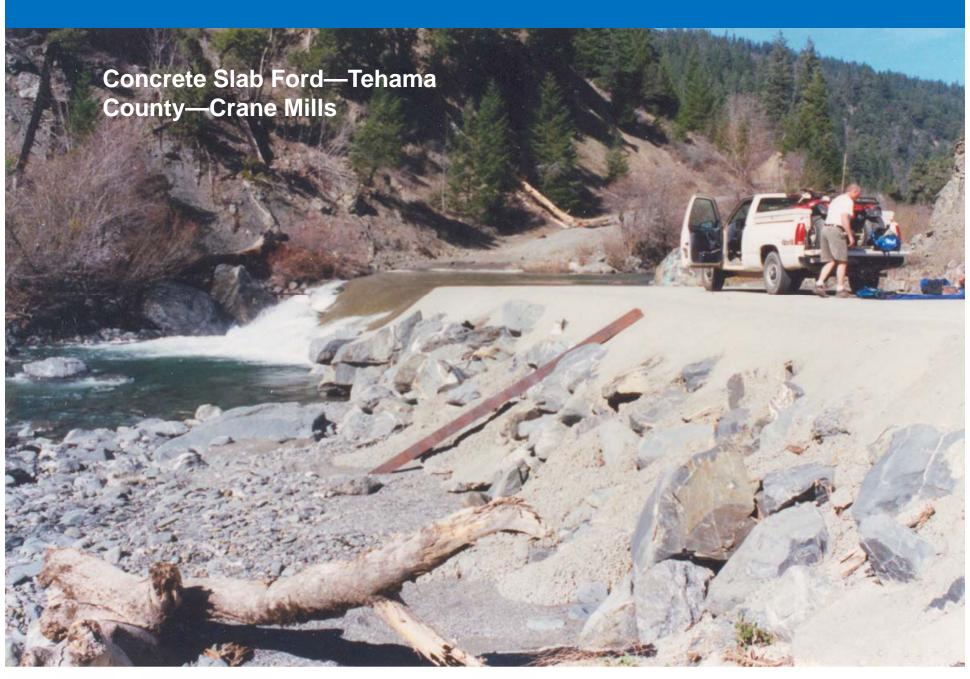


Rock Ford Design Specifications

Tim Best, CEG







Thomes Creek "The Slab." Brett Rohrer DFG



Diagram of a Vented Ford Crossing (Keller and Sherar 2003)



b. Improved (Vented) Ford with Culvert Pipes in a Broad Channel

Note that armoring must extend to the 100 yr High Water Level on either side of the pipes.

Comparison of Crossing Types Impacts on Water Quality

- Culverts: higher catastrophic failure risk; lower chronic sediment input.
- Fords: lower catastrophic failure risk;
 higher chronic sediment input.
- Chronic Water Quality Effects from sediment entry:
 - Highest: fords and culverts
 - Lowest: bridges

E. Temporary Crossings

Pro's

- Little impact to stream channel if designed and implemented correctly.
- Almost no chance for failure.
- Complete fish passage.
- Relatively inexpensive.
- Required for temporary roads (pulled by October 15th).

Con's

- Higher sediment input, especially if built incorrectly.
- High sediment input if removed incorrectly.

"Spittler crossing" typical

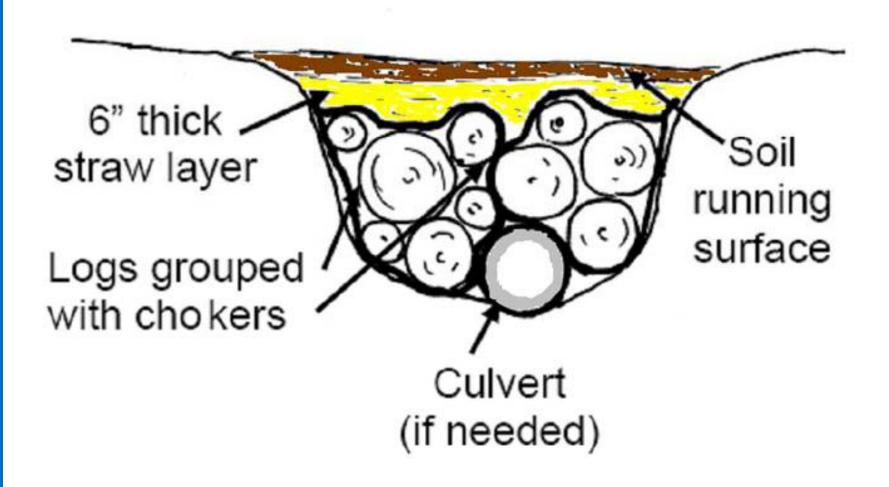


Image: T. Spittler, CGS (retired)





Temporary 12 inch plastic pipe

Santa Cruz County







Crossing Crosswalk

Watercourse Type	<u>Crossing Alternatives</u>
Class I fish-bearing	 Temporary crossing; Bridge; 3. Open-bottom arch; 4. Pipe arch culvert
Class II non-fish bearing perennial	Culvert or Temporary Crossing
Class II non-fish bearing intermittent (headwater)	Culvert or Rock ford (or Temporary crossing)
Class III ephemeral	Rock ford or Culvert (or Temporary crossing)

PREFERRED ALTERNATIVES FOR FISH-BEARING CROSSINGS

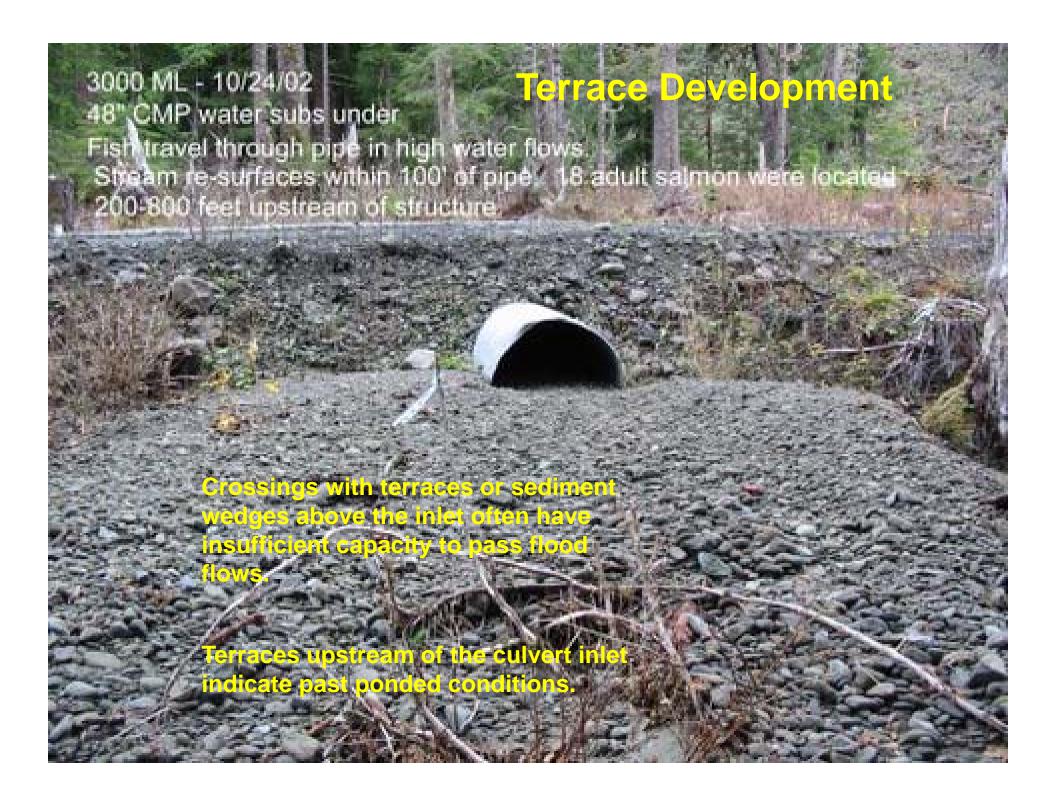
The following alternatives and structure types should be considered in order of preference (NMFS 2001):

- 1. **Nothing** Road realignment to avoid crossing the stream.
- 2. <u>Bridge</u> spanning the stream to allow for long term dynamic channel stability.
- 3. Bottomless arch, embedded culvert design, or ford.
- 4. **Non-embedded culvert** this is often referred to as a hydraulic design, associated with more traditional culvert design approaches limited to low slopes for fish passage.

IV. What Can Go Wrong at a Crossing?

(short answer—LOTS!)







Culverts Blocked by Woody Debris

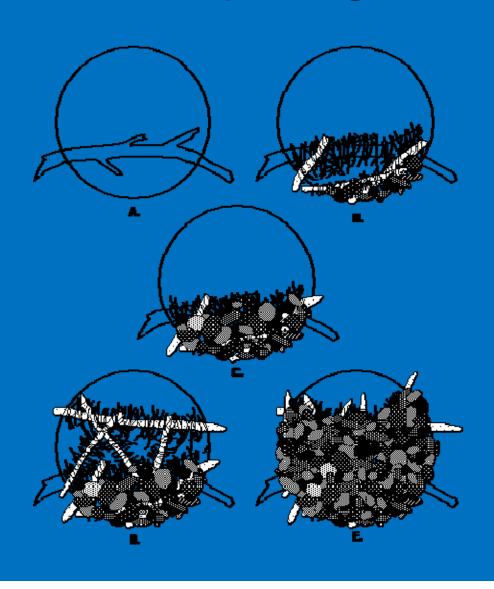
Photo: Wopat, CGS



Photo: Wilson, CVRWQCB

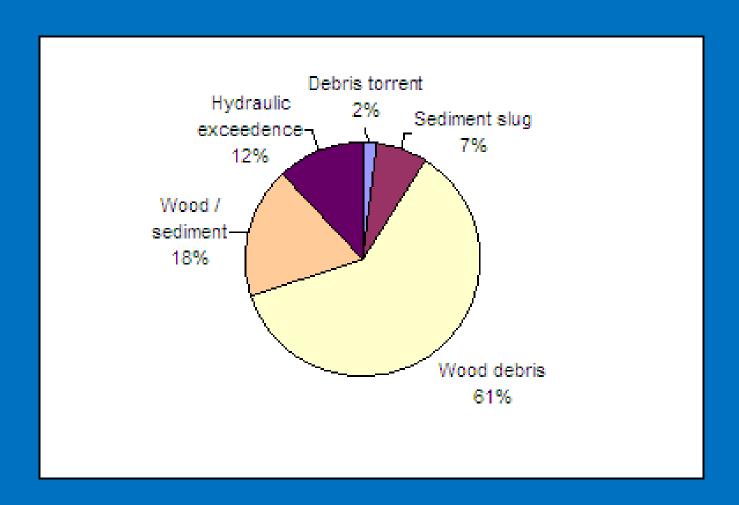


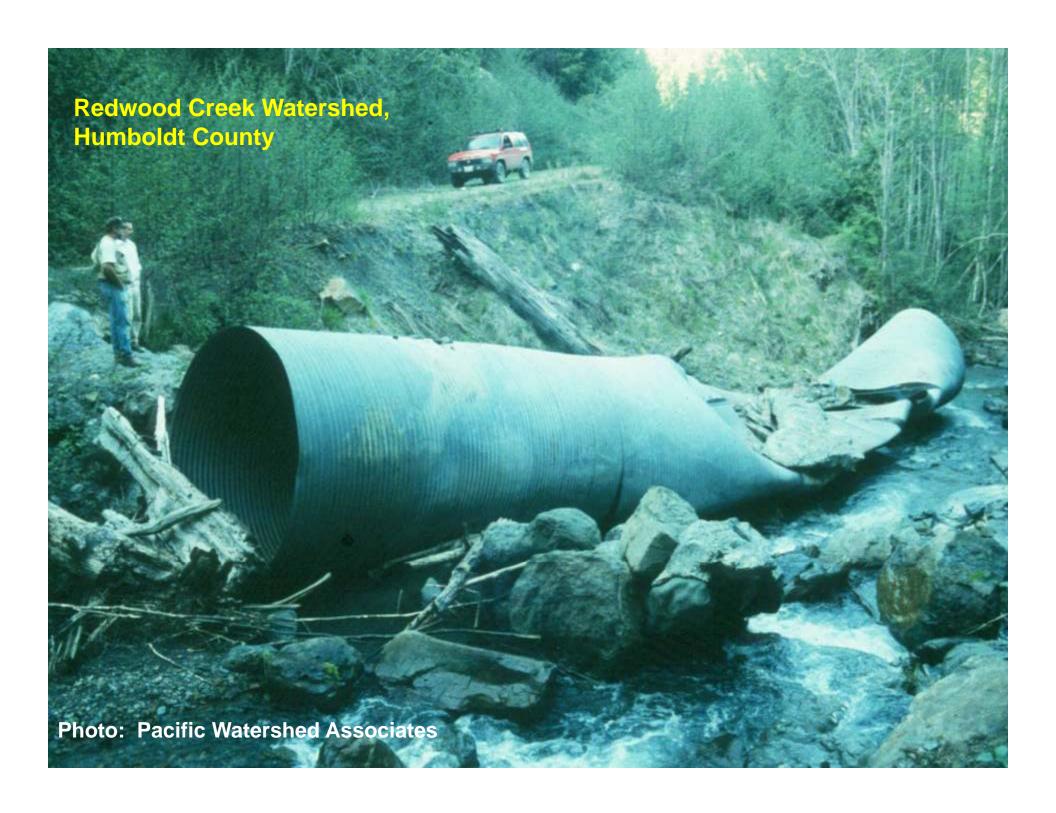
Typical Woody Debris Lodging at the Culvert Inlet (Flanagan 2004)





Failure Mechanisms for Culverts Along Forest Roads in Northwest CA Associated with Storms < 12 RI (Flanagan 2004)





Humboldt County Crossing—Inlet Blocked

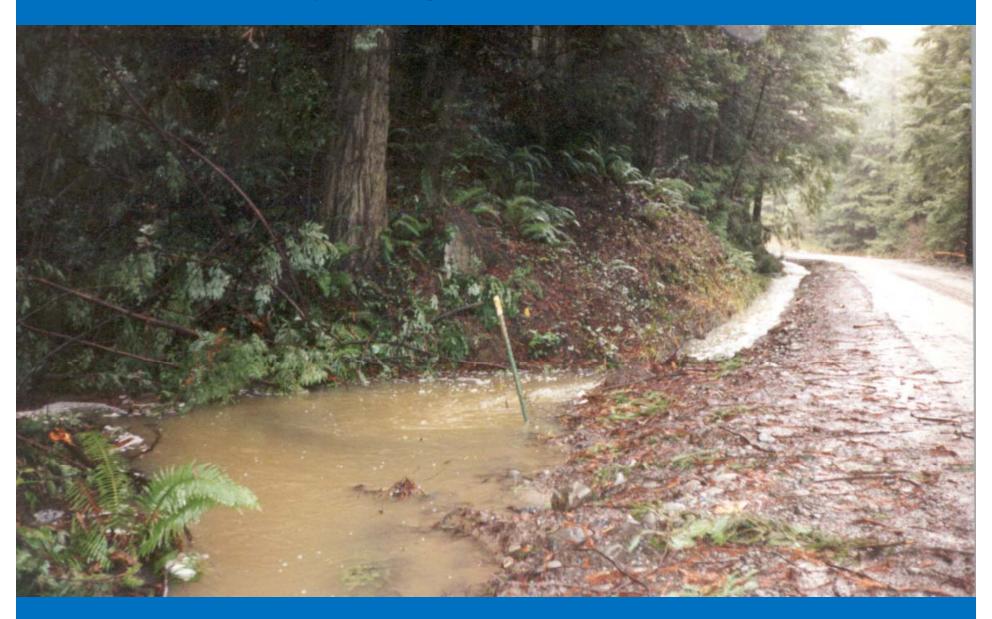
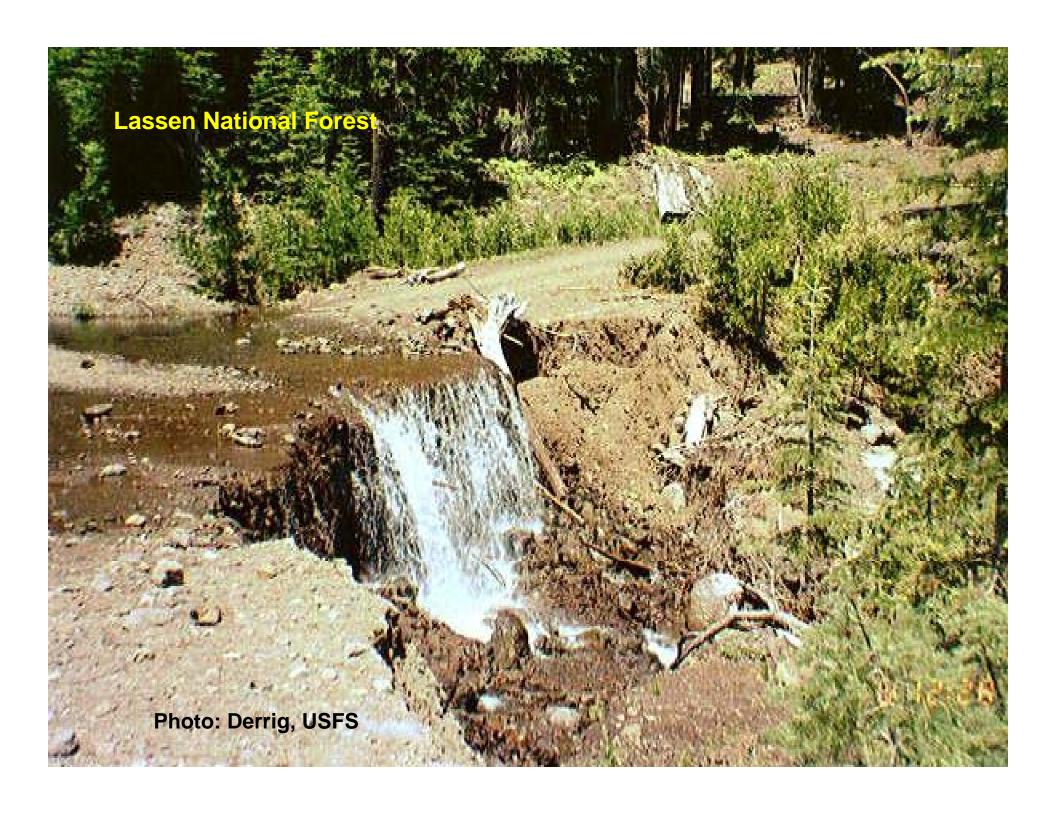


Photo: Scanlon, CAL FIRE





Stream
Diversion
from
Blocked
Culvert Inlet

Photo: Pacific Watershed Associates

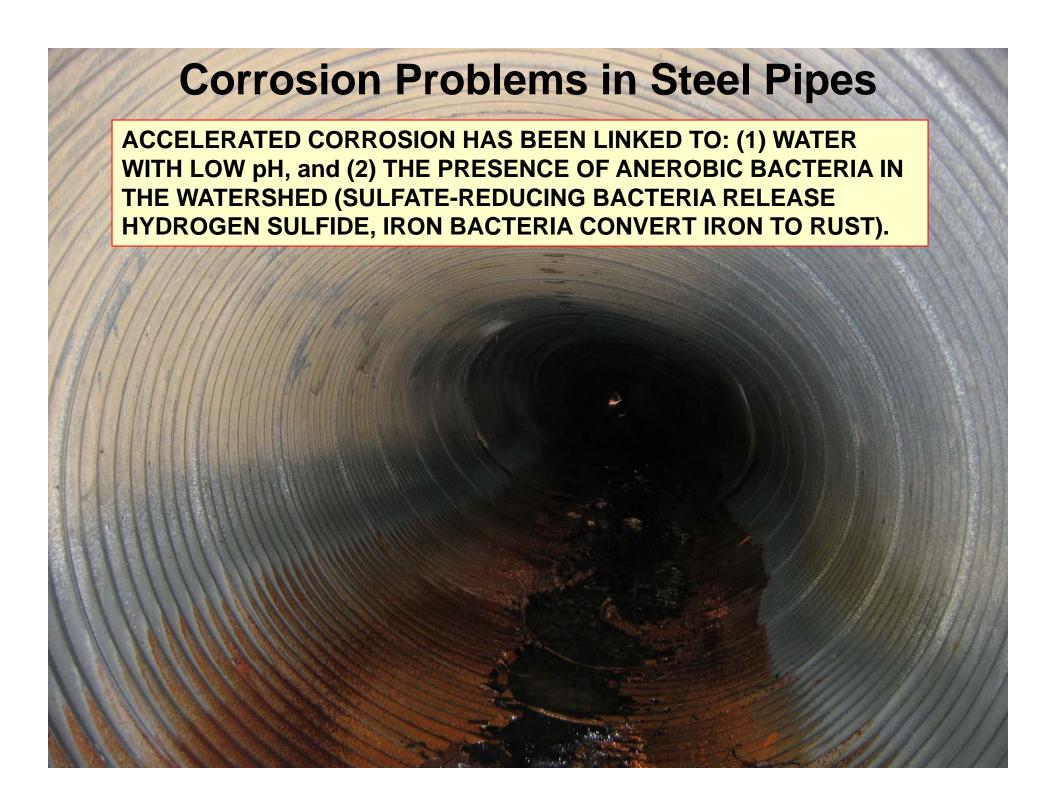
Example of Stream Diversion Gullies — Redwood Creek Watershed





Photos: Bundros, RNSP (retired)





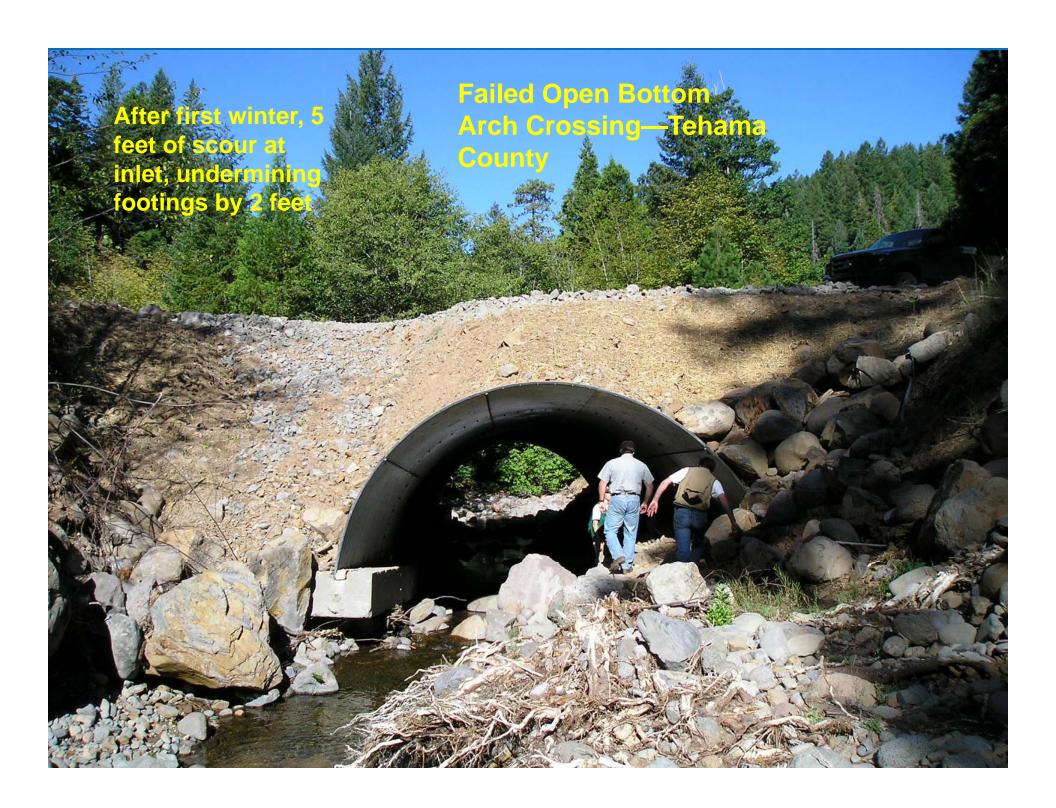


Culvert Crushing and Plugging









Failed Rock Ford Crossing—Undersized Rock





V. Proper Design and Construction for New or Reconstructed Culverted Stream Crossings*

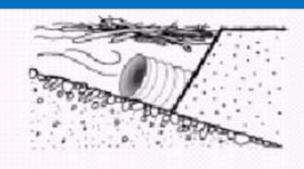
[In addition to Proper Design for 100 yr Flood Flow]

If you Modify a Stream's Bed or Banks, you must First Notify the California Department of Fish and Game and Obtain a Streambed Alteration Agreement (or 1600 Agreement)

1. Culverts Should Not Pond Water

(Furniss and Others 1998)







HW/D < 1.0 (suggest 0.67)

GOOD

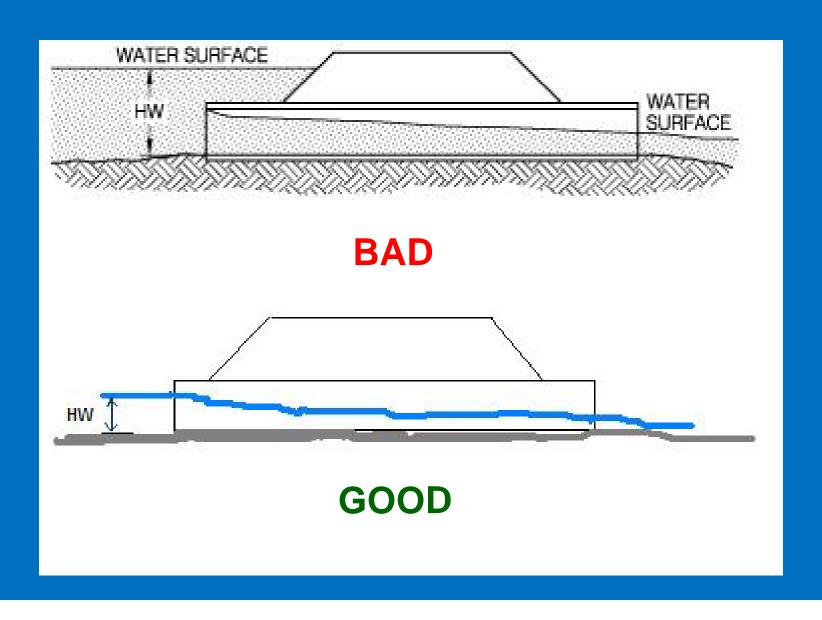
Reduced Plugging Hazard

HW/D > 1.0

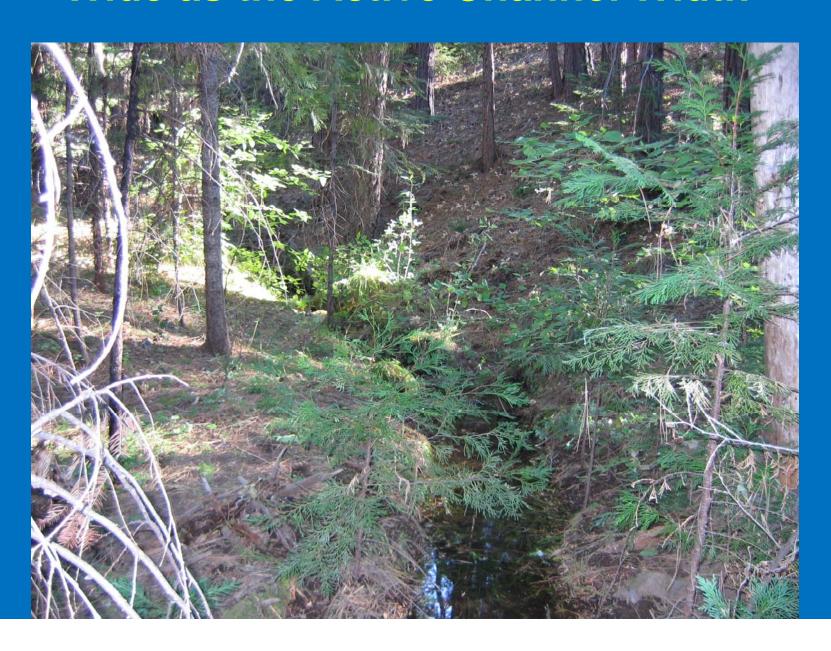
HW/D > 1.0

BAC

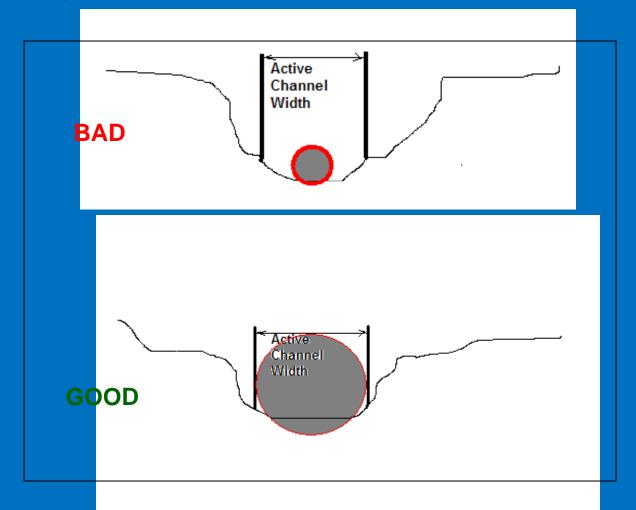
Culverts Should Not Pond Water



2. Utilize Culverts as Wide or Nearly as Wide as the Active Channel Width

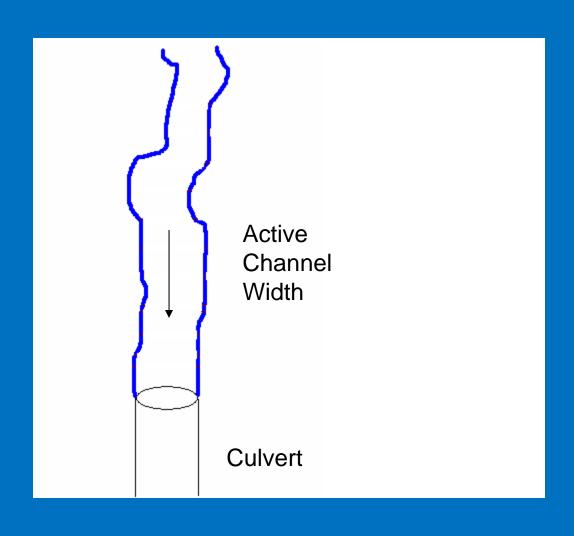


Channel Width vs. Culvert Inlet Diameter

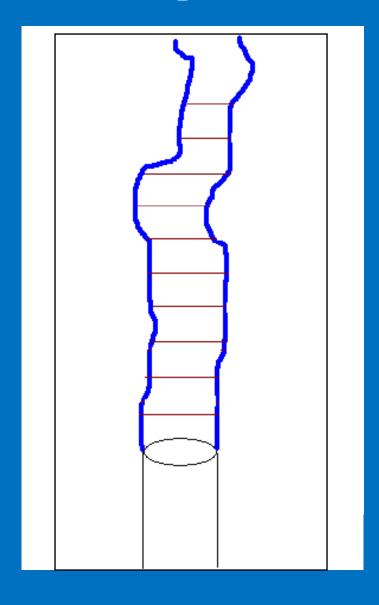


Small pipe in a wide channel = high risk of plugging by woody debris

Channel Width vs. Culvert Inlet Diameter



Determining Active Channel Width



Make 10 systematic measurements of active channel width.

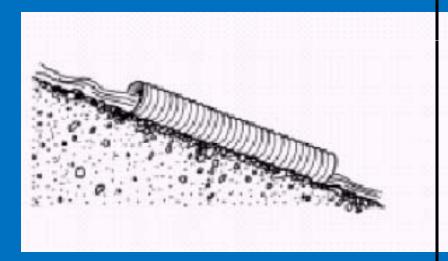
20 ft intervals, beginning 20 ft above the pipe inlet.

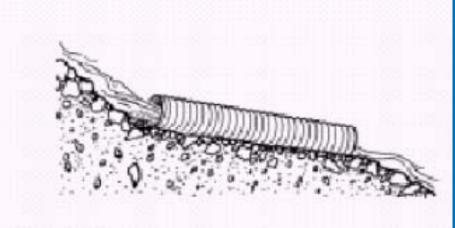
Calculate average width.



3. Culvert Should Maintain Channel Grade to Avoid Bedload Accumulation

(Furniss and Others 1998)



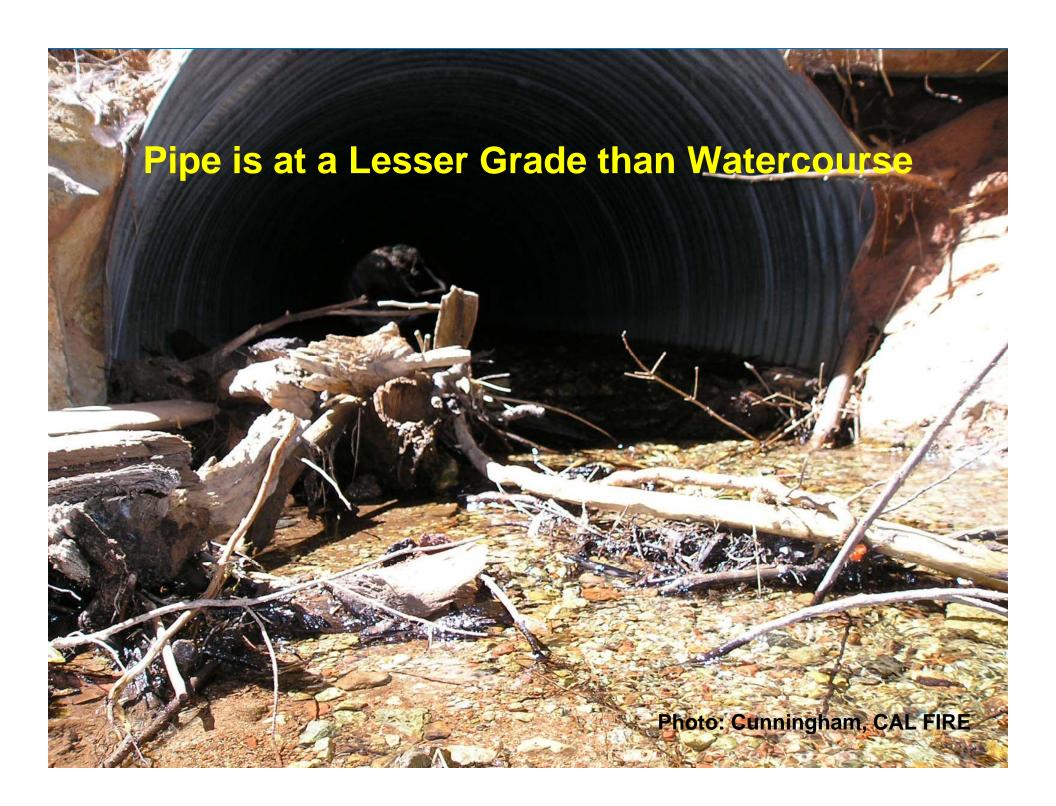


GOOD

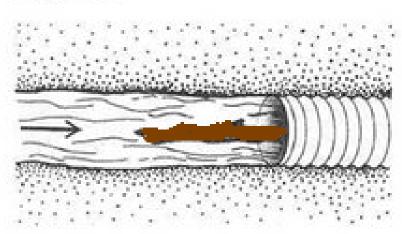
Reduced Plugging Hazard

BAD

Pipe slopes of <3% may be prone to bedload sediment accumulation.

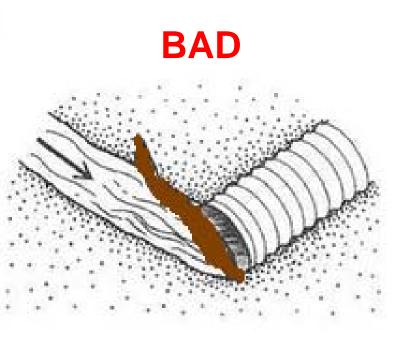


4. Culverts Should be Placed on the Same Alignment as Natural Stream Channel (Furniss and Others 1998)



GOOD

Reduced Plugging Hazard



Misaligned Crossing

After Construction

After One Winter



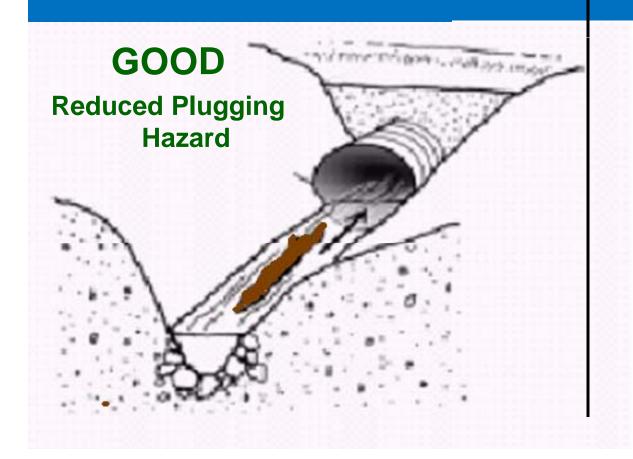


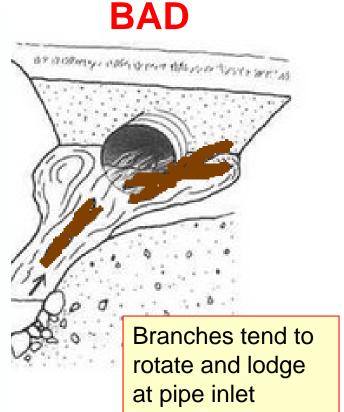
Misalignment can result in Road Fill/ Bank Erosion!

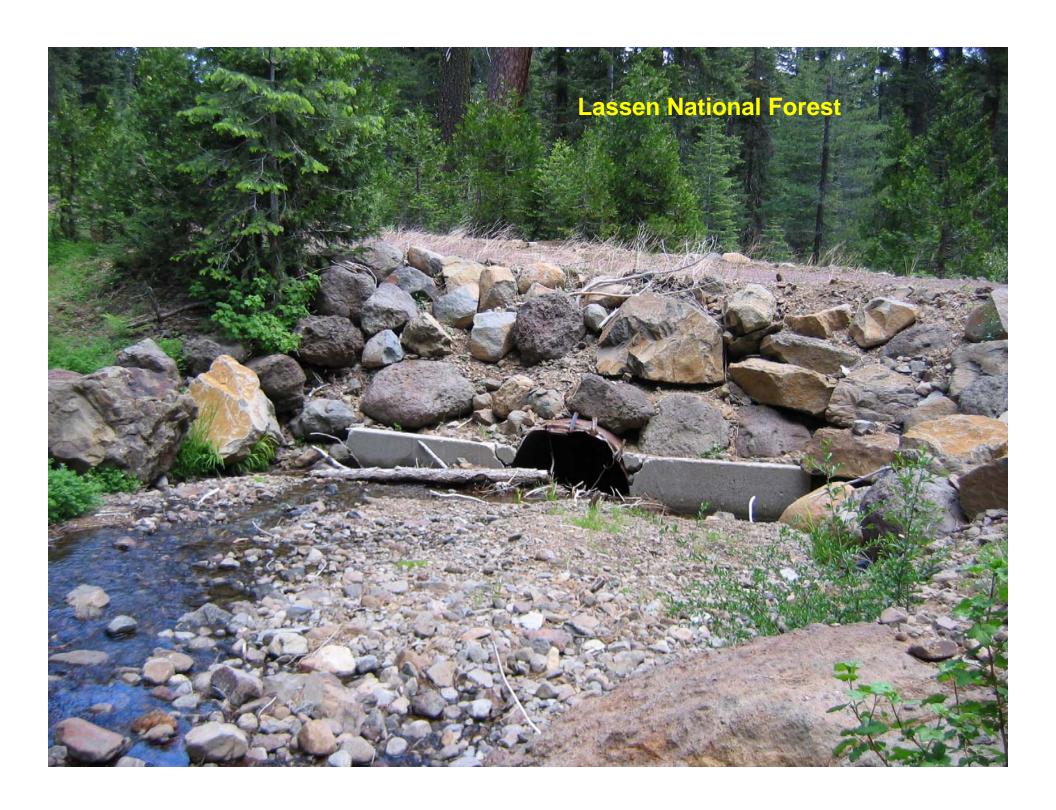
Photos: Harris, UCB (retired)

5. Culverts Should Not Create Wide Areas Near the Pipe Inlet

(Furniss and Others 1998)











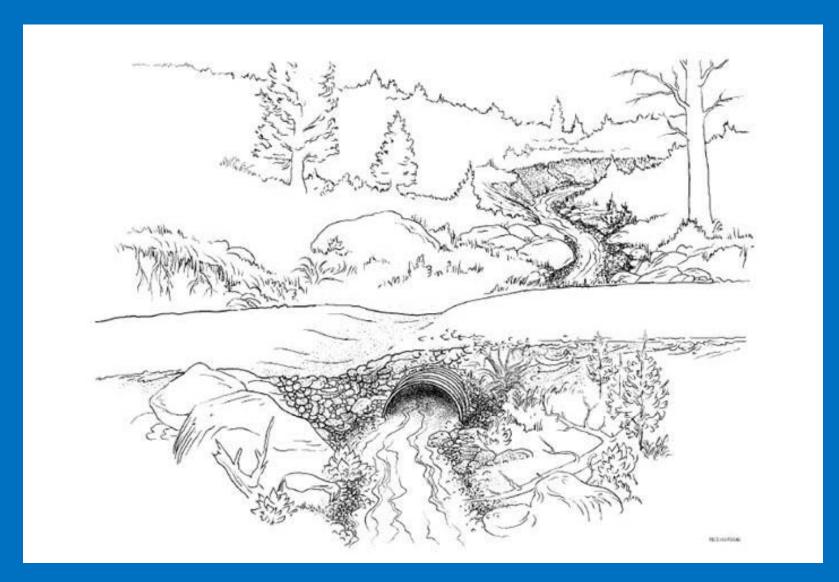
6. Single Large Pipe—Not Multiple Pipe Barrels

 Installing multiple pipes is a bad strategy for passing woody debris.



Use Single
Large Pipe to
Minimize
Plugging
Potential!

7. Critical Dip Installed to Prevent Diversion Potential—Required BOF Rule since 1990



Goal: Keep water in its natural drainage!

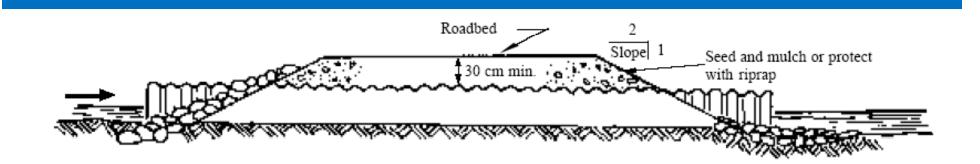
8. Culvert Installation-Depth of Fill Over Culvert

• Cover the top of metal pipes with fill to a depth of at least 12 inches to prevent pipe crushing by log trucks (or at least 1/3 of pipe diameter for larger pipes).

 Minimum cover of 24 inches for concrete pipes.

Typical Culvert Installation with a Projecting Inlet

(Keller and Sherar 2003)



- > At least 12 inches of fill over pipe
- > Rock armored inlet and outlet to prevent erosion of the fill

8. Culvert Installation (cont'd)

- Both ends should extend at least one foot beyond the edge of the fill material.
- Backfill material around the pipe should be moist, well-graded soil with up to 10% fines and free of rocks (avoid non-cohesive uniform fine sand).
- Backfill material should be well compacted (compacted in 6 inch lifts or layers).
- Both the culvert inlet and outlet should be armored with rock to protect against erosion.

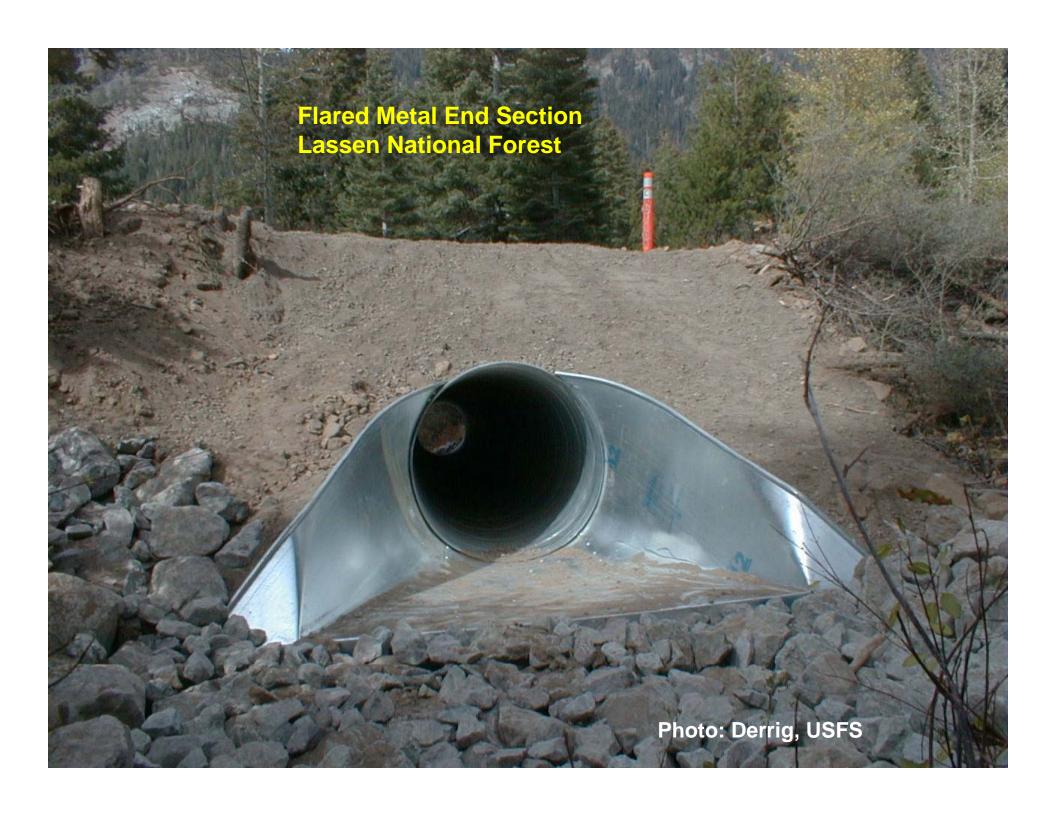


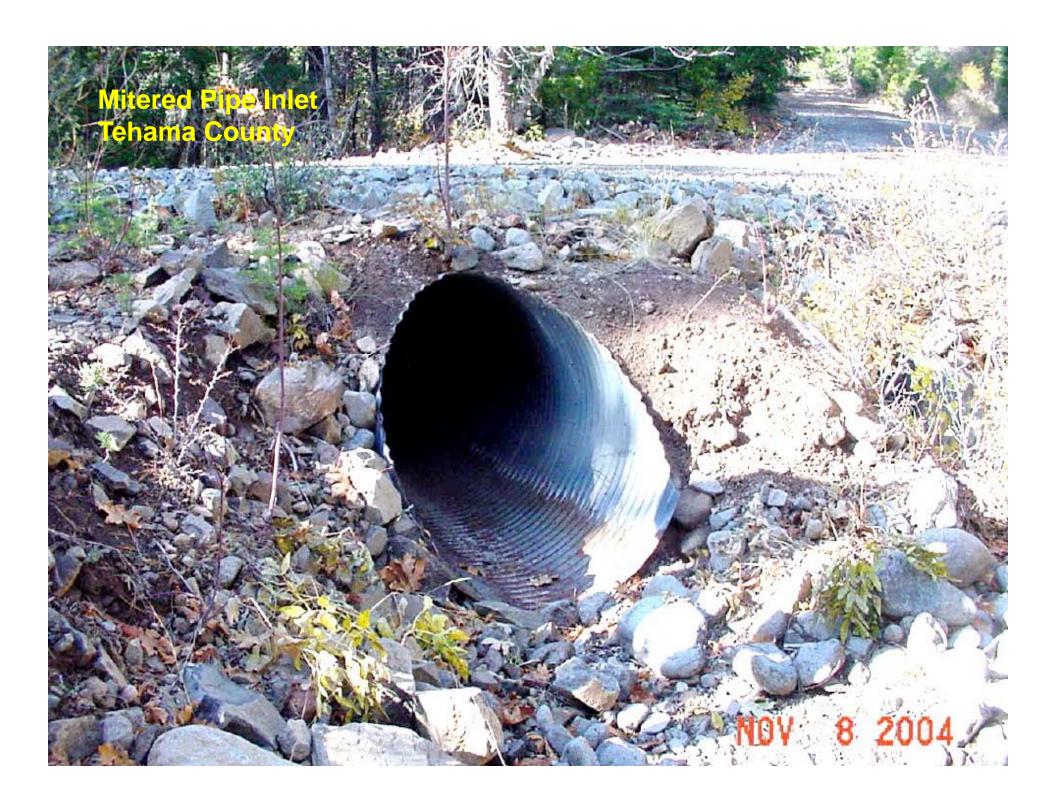






8. Additional Techniques to Reduce Crossing Problems





Mitered Inlets

- A mitered culvert is formed when the culvert is cut to conform with the plane of the embankment slope.
- Beveled inlets reduce wood blockage and plugging.
- Bevel the inlet edge to increase flow efficiency and reduce pipe size by <10%.
- "Mitered ends are much less likely to be plugged by ice, debris, or beaver" (Wisconsin Transportation Bulletin).





Costs of Additional Techniques

- Simple Trash Rack (Fence Posts)
 - <\$50 (but requires abundant maintenance!)</p>
- Bevel (Mitered) End Section
 - **-~\$50 \$100**
- Flared Metal End Section
 - 36 inch pipe -- ~\$500
 - 48 inch pipe -- ~\$1,100
 - 60 inch pipe -- ~\$2,000

VI. Summary (Take Home Messages)

- Stream crossings are high risk locations for sediment entry and road travel limitations (20% have problems).
- In the Sierra Nevada, most large floods are the result of rain-on-snow events (big—once ~ every 10 yrs).
- Crossings need to be built correctly for large flood flows (100-yr return interval), as well as sediment and wood passage.
- Numerous types of crossings are available (no one "right answer")—pick the type that fits the features of the landscape, the maintenance that will be possible, and the legal requirements (e.g., fish passage).

VI. Summary (Take Home Messages)

- Numerous problems can occur at all types of crossings, indicating that frequent monitoring observations are needed, with upgrading performed as required.
- Simple guidelines are available for culverted crossings to ensure that new or reconstructed crossings have reduced risk of failure and WQ impact.
- Crossings must be <u>maintained</u> over time—you cannot install them and expect them to function properly without proper upkeep—clean/maintain on a regular basis and during/after large storm events.

VII. Stream Crossing References

- Cafferata and others 2004 -- Designing Watercourse Crossings for Passage of 100-year Flood Flows, Wood, and Sediment
- Clarkin and others 2006 -- Low-Water Crossings: Geomorphic, Biological, and Engineering Design Considerations
- Flanagan 2004 Woody Debris transport Through Low-Order Stream Channels of northwest California—Implications for Road-Stream Crossing Failure
- Flanagan and others 1998 -- Methods for Inventory and Environmental Risk Assessment of Road Drainage Crossings
- Flanagan and Furniss 1997 -- Field Indicators of Inlet Controlled Road Stream Crossing Capacity
- Furniss and others 1998 -- Response of Road Stream Crossings to Large Flood Events in Washington, Oregon, and Northern California
- Furniss et al. 1997– Diversion Potential at Road-Stream Crossings
- Keller and Sherar 2003, Chapter 8 -- Culvert Use, Installation, and Sizing
- Merrill and Casaday 2001—BMPs for Culvert Replacement
- Weaver and Hagans 1994—Forest and Ranch Roads (Chapter V-Drainage, VI-Construction)
- ODF 2002 -- Determining the 50-year Peak Flow and Stream Crossing Structure Size for New and Replacement Crossings